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RELIABILITYOFREINFORCEDCONCRETEBUILDINGSDURINGCONSTRUCTION

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Abstract: The safety analysis of reinforced concrete buildings during construction should be based on the comprehensive understanding of loads, load effects, structural resistance, and available safety index of the structure. This paper analyzes the characteristics and probabilistic models of resistance, loads, and load ef-fects. A method was developed to calculate the probability of failure based on Monte Carlo simulation and models proposed in previous articles. Construction examples were used to analyze the influence of live load on the probability of failure. The results show that when the live load increases, the maximum probability of failure increases with acceleration. The results suggest that the construction live load should be carefully addressed during construction.

Keywords:reinforcedconcretebuildings;duringconstruction;loadredistribution;failureprobability

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Introduction

The characteristics of resistance, load, load effect, andfailure mode should be taken into consideration whenanalyzing the structural safety of reinforced concretebuildingsduringconstruction, which were referred to a stemporary or time dependent structures.

Lew^[1] identified three different types of load com-binationsduring different construction stages, and pointed out that the load distributions on different floorslabs must be considered. Karshenas and Ayoub col-lected construction live load data from 24 constructionsites^[2] and they analyzed the data and converted the live loads into an equivalent uniform distribution load (EUDL) using the influence surfacement hod to make the construction safety analyses more convenient. Mesh sampling of the construction loads on a floor plane showed that the Weibull, log-normal, exponential, and gamma distribution probability models were acceptable, with the Weibull distribution as the best. The construction loads are presented as a composite distribution that is a combination of a zero load and acontinuous nonzero load distribution. The EUDL prob-ability model and the load effects can be described by the same distribution.

LOADS AND LOAD COMBINATIONS

During the construction of reinforced concrete build-ings, each construction cycle can be divided into threemajorstages:thecastingstage,thecuringstage,andthe shore removal stage. During the casting and shoreremovalstages,theloads on thestructurememberschange suddenly. During the curing stage, a transitionappears between the casting and shore removal stages,and the loads and properties of the structure memberschangesmoothly. Many researchers analyzed loads during construction but their conclusions differed due to different datasets and different dataprocessing methods. Table 1 lists various probabilistic models and parameters that have been used in the literature. In this paper, the loads are defined as follows.

DEAD LOAD

Dead loads include the weight of the reinforced con-crete members, the formwork, and the shores. For theweight of the reinforced concrete members, the ratio of the mean value to the standard value of the weight is assumed to be 1.06, and the coefficient of variation is 0.074. With a normal distribution [18]

OCCASIONAL LOAD

In addition to normal loads, specific construction ac-tions may induce occasional loads on the temporary structures. These occasional loads, such as heavy machinery or extraordinarily heavy material put on the temporary structures. These occasional loads, such as heavy machinery or extraordinarily heavy material put on the temporary structures.

LOAD EFFECT

The construction load effect can be obtained by analyzingtemporarystructuressubjecttoconstructionloads [16]. However, the construction load effect is verycomplex because the structural configuration and theconstructionmaterial characteristics are constantly

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MATERIAL PROPERTY UNCERTAINTIES

Constructionmaterialpropertyuncertainties result from the variation of the structural performance of the development and the probability distribution of the concrete strength during curing were investigated by measuring concrete strength of 275-cubic concretes amples fabricated for five different design strengths at 3 d, 7 d, and 28 d. All the samples were grouped in ac-cordance with their design strengths and age with their strength converted to in-placestrengths according to the Chinese concrete code [24], as shown in Table 3. The result was then used in the computational examples in this paper with the concrete strength assumed to follow an ormal distribution.

Table3Probability distribution parameters for initial concrete strength

Typeofcement	Age (d)	Mean value(MPa)	Standarddeviation(MPa)	Variationcoefficient
Portland-slagcement				
C10	3	7.67	2.64	0.34
	7	13.11	3.47	0.26
	28	20.17	4.84	0.24
C15	3	10.03	2.66	0.27
	7	15.84	3.52	0.22
	28	25.31	5.78	0.23
C20	3	12.14	2.71	0.22
	7	20.43	2.61	0.13
	28	29.48	3.85	0.13
C25	3	14.65	2.96	0.20
	7	23.68	2.92	0.12
	28	34.83	4.55	0.13
C30	3	17.59	3.31	0.19
	7	26.78	3.31	0.12
	28	40.26	1.29	0.03

CONCLUSIONS

analysis A new structural model used to develop was astructuralreliabilityanalysismethodtopredicttheprobability of failure for reinforced concrete buildingsduring construction. The analytical method was alsoused to analyze the impact of the live load on the prob-ability of failure. The results show that when the liveload increases, $P_{f,max}$ increases with acceleration. The construction live loadshould be carefully addressed during construction.

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